

EDC-7 Innovation : Enhancing Performance with Internally Cured Concrete (EPIC²)

HIGH PERFORMANCE INTERNALLY CURED CONCRETE (HPIC)

NJSTIC Meeting

April 30th, 2025

Samer Rabie, Project Manager, DPM Team D

Jess Mendenhall, PE, Structural Engineer, Structural Engineering Services

AGENDA



DEFINE THE
PROBLEM &
OBJECTIVE



INTRODUCE
INTERNAL
CURING



DISCUSS THE
IMPLEMENTATION
PLAN



NEXT STEPS
& GOALS

The State Transportation Innovation Council (STIC)

NJ State Transportation Innovation Council (STIC)

EXECUTIVE TEAM

Manages STIC Operation
Point for reporting to Assistant Commissioner & FHWA
Coordinates with all groups

Innovation Advisory Team

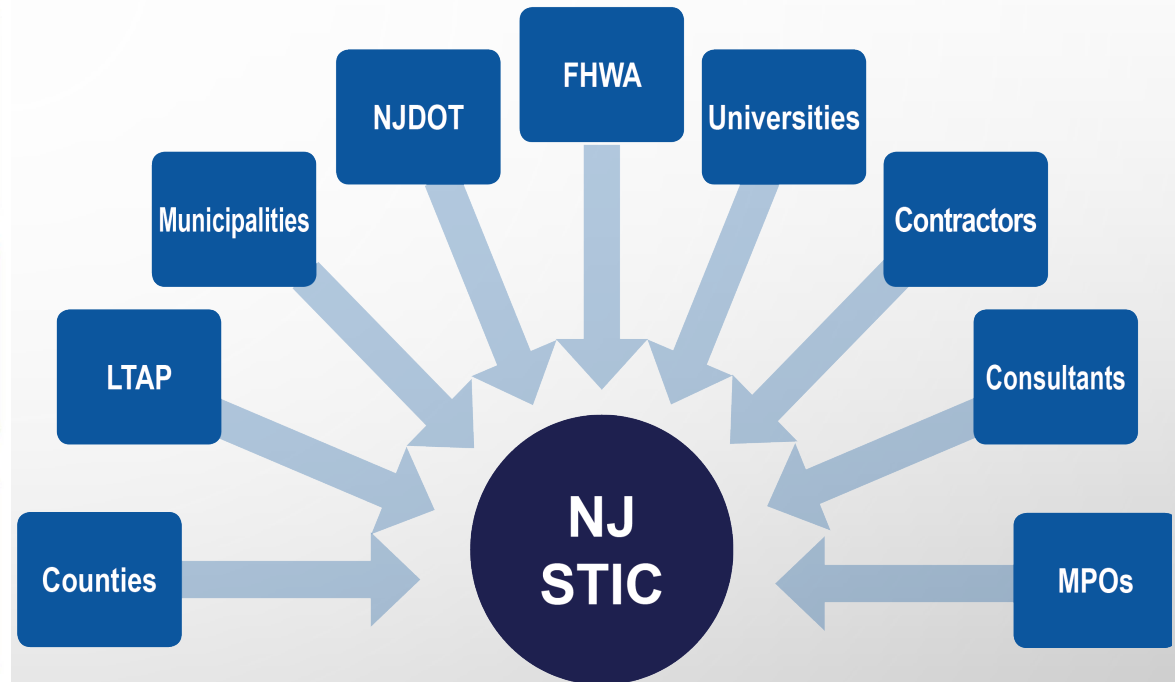
Core Innovation Area
Infrastructure Preservation

Core Innovation Area
Mobility & Operations

Core Innovation Area
Organizational Support & Improvement

Core Innovation Area
Planning & Environment

Core Innovation Area
Safety



***“Identify and rapidly deploy proven yet underutilized innovations...
shorten the project delivery process, enhance roadway safety, reduce traffic congestion, or integrate automation.”***

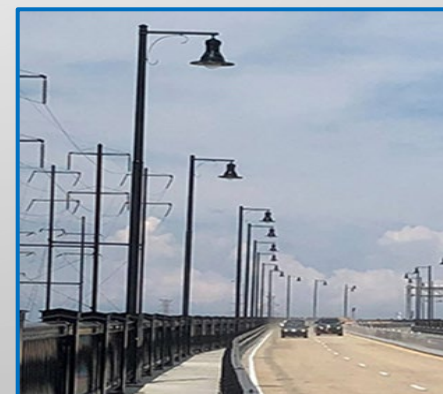
- **EDC 1 (2011-2012) :**
PREFABRICATED BRIDGE ELEMENTS & SYSTEMS (PBES)
- **EDC 2 (2013-2014) :**
ACCELERATED BRIDGE CONSTRUCTION
- **EDC 3 & 4 (2015-2018) :**
ULTRA HIGH PERFORMANCE CONCRETE (UHPC) FOR PBES
- **EDC 6 (2021-2022) :**
UHPC FOR BRIDGE PRESERVATION & REPAIR
- **EDC 7 (2023-2024):**
ENHANCING PERFORMANCE WITH INTERNALLY CURED CONCRETE (EPIC²)

Core Innovation Area

**Infrastructure
Preservation**

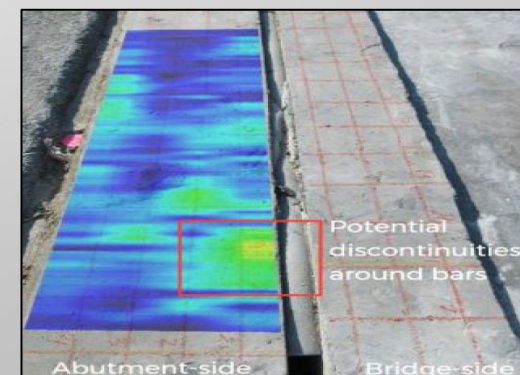
The State Transportation Innovation Council (STIC)

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UHPC FOR BRIDGE PRESERVATION & REPAIR
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ENHANCING PERFORMANCE WITH INTERNALLY CURED
CONCRETE (EPIC²)



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ENHANCING PERFORMANCE WITH INTERNALLY CURED
CONCRETE (EPIC²)



The State Transportation Innovation Council (STIC)

- EDC 1 (2011-2012): **Institutionalized**
PREFABRICATED BRIDGE ELEMENTS & SYSTEMS (PBES)
- EDC 2 (2013-2014): **Institutionalized**
ACCELERATED BRIDGE CONSTRUCTION
- EDC 3 & 4 (2015-2018): **Institutionalized**
UHPC FOR PBES
- EDC 6 (2021-2022): **Assessment**
UHPC FOR BRIDGE PRESERVATION & REPAIR



- **EDC 7 (2023-2024):**

**ENHANCING PERFORMANCE WITH INTERNALLY CURED CONCRETE
(EPIC²)**



HIGH PERFORMANCE CONCRETE (HPC)

HPC Section 903.05 :

- Low w/cm ratio : 0.40
- Large amount of Pozzalone or SCM's (fly ash, slag, silica fume)
- Improved durability properties

However,

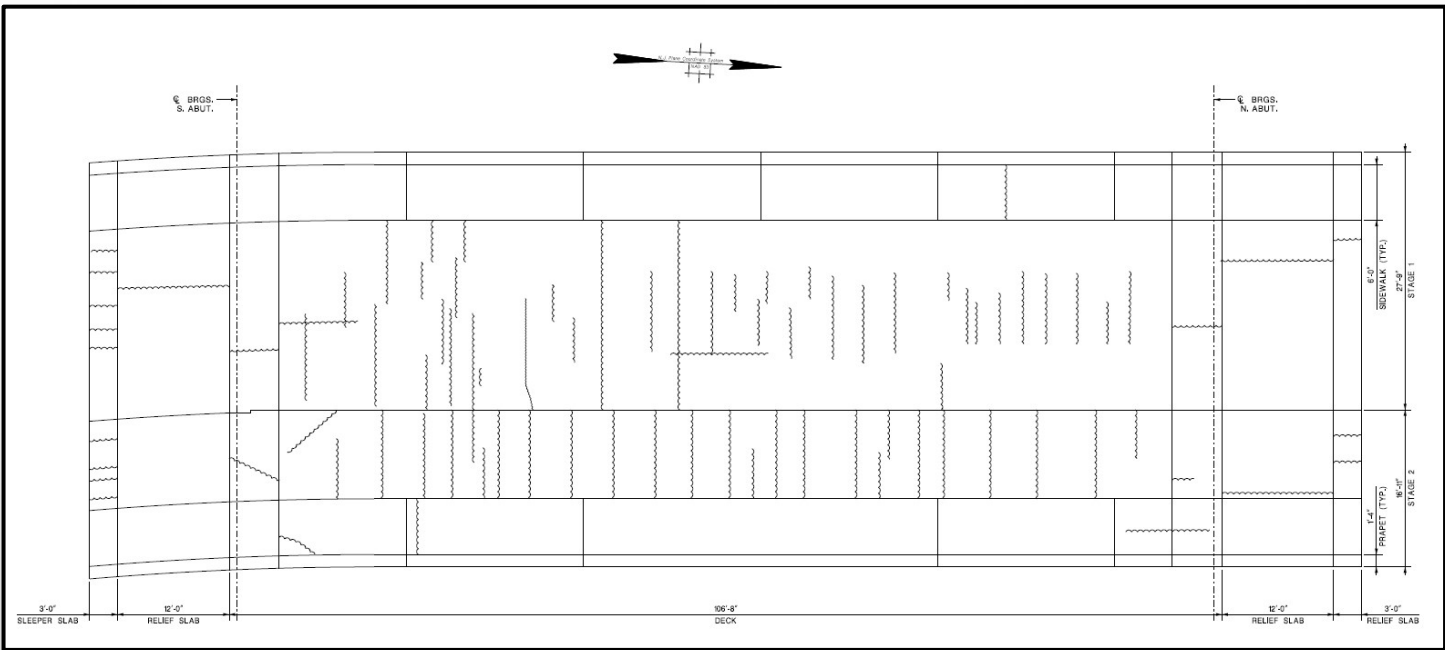
- Increased susceptibility of cracking
- Undermined durability improvements
- Most HPC Bridge Decks experience shrinkage cracking
- Sealing of the cracking recommended



Curing Section 507.03.02.J :

- Within 30 min of texturing, apply wet burlap and white polyethylene sheeting for at least 14 days.
- Reduces Shrinkage, Cracking & Water movement.

HPC BRIDGE DECK TRANSVERSE CRACKING



New Jersey Department of Transportation

1035 Parkway Avenue, PO Box 600, Trenton, New Jersey 08625-0600

Baseline Document Change Announcement



ANNOUNCEMENT: BDC21S-05

DATE: January 06, 2023

SUBJECT: Concrete Deck Repair
- Revision to the 2019 Standard Specifications for Road and Bridge Construction, Subpart 507.02.01 and Subsection 507.04, & addition of Subpart 507.03.08

Subpart 507.02.01 and Subsection 507.04 have been revised & new Subpart 507.03.08 has been added to the 2019 Standard Specifications for Road and Bridge Construction in order to allow the RE to direct if and where to apply BRIDGE DECK AND BRIDGE APPROACH SEALING.

	QTY	UNITS	UNIT PRICE	SUBTOTAL	MARK UP	SUB TOTAL
MATERIAL						
FURNISH SIKAGARD -705L (2 COATS APPROX. 11,000SF/CT) - 5 GAL PAIL	20	EA	\$ 375.00	\$ 7,500.00	\$ 1,875.00	\$ 9,375.00
FURNISH PUMP SPRAYERS	2	EA	\$ 180.00	\$ 360.00	\$ 90.00	\$ 450.00
LABOR (6 MAN CREW, 10 HR SHIFTS) 2 SHIFTS TO PREP, 2 SHIFT TO APPLY						
LABOR FOREMAN	40	HR	\$ 127.36	\$ 5,094.40	\$ -	\$ 5,094.40
LABORER	200	HR	\$ 124.79	\$ 24,958.00	\$ -	\$ 24,958.00
EQUIPMENT PER SHIFT						
FOREMAN TRUCK	40	HR	\$ 24.26	\$ 970.40	\$ -	\$ 970.40
LIGHT TOWER	40	HR	\$ 14.82	\$ 592.80	\$ -	\$ 592.80
GENERATOR	40	HR	\$ 8.80	\$ 352.00	\$ -	\$ 352.00
COMPRESSOR	40	HR	\$ 16.68	\$ 667.20	\$ -	\$ 667.20
PRESSURE WASHER	40	HR	\$ 5.79	\$ 231.60	\$ -	\$ 231.60
TMA	40	HR	\$ 41.27	\$ 1,650.80	\$ -	\$ 1,650.80
BOX TRUCK	40	HR	\$ 27.42	\$ 1,096.80	\$ -	\$ 1,096.80
ARROW BOARD	40	HR	\$ 2.43	\$ 97.20	\$ -	\$ 97.20
GRAND TOTAL						\$ 45,536.20

TYPES OF SHRINKAGE CRACKS

Type	What is happening	When	What is driving this	Mitigation strategies
Plastic	Water leaving the concrete <u>before setting</u>	First few hours	Bleeding & evaporation rate	Use fibers, curing membranes, foggers, etc..
Drying	Water leaving the concrete after setting	Weeks and months	Relative humidity of the environment	External curing, aggregate choice, minimize water, shrinkage reducing admixtures, reinforcement, expansive cements
Thermal	Concrete temperature changing	First few days and on-going	Environmental temperatures swings and high heat of hydration	Minimize heat of hydration, low thermal expansion aggregates
Autogenous (Chemical)	Water consumed by hydration of cementitious materials <u>after setting</u>	First few days	Self-desiccation of cementitious materials (insignificant when $w/c > 0.42$)	Minimize self desiccation and rate of hydration (lower cement content), reinforcement, and <u>INTERNAL CURING</u>

PROBLEM

Early-age Cracking (within 60 days) in HPC Bridge Decks and the resulting Reduction in Service Life.

OBJECTIVE

Reduce Autogenous Shrinkage and the resulting Transverse Cracking in new HPC Bridge Decks.

EDC-7:

ENHANCING PERFORMANCE WITH INTERNALLY CURED CONCRETE (EPIC²)

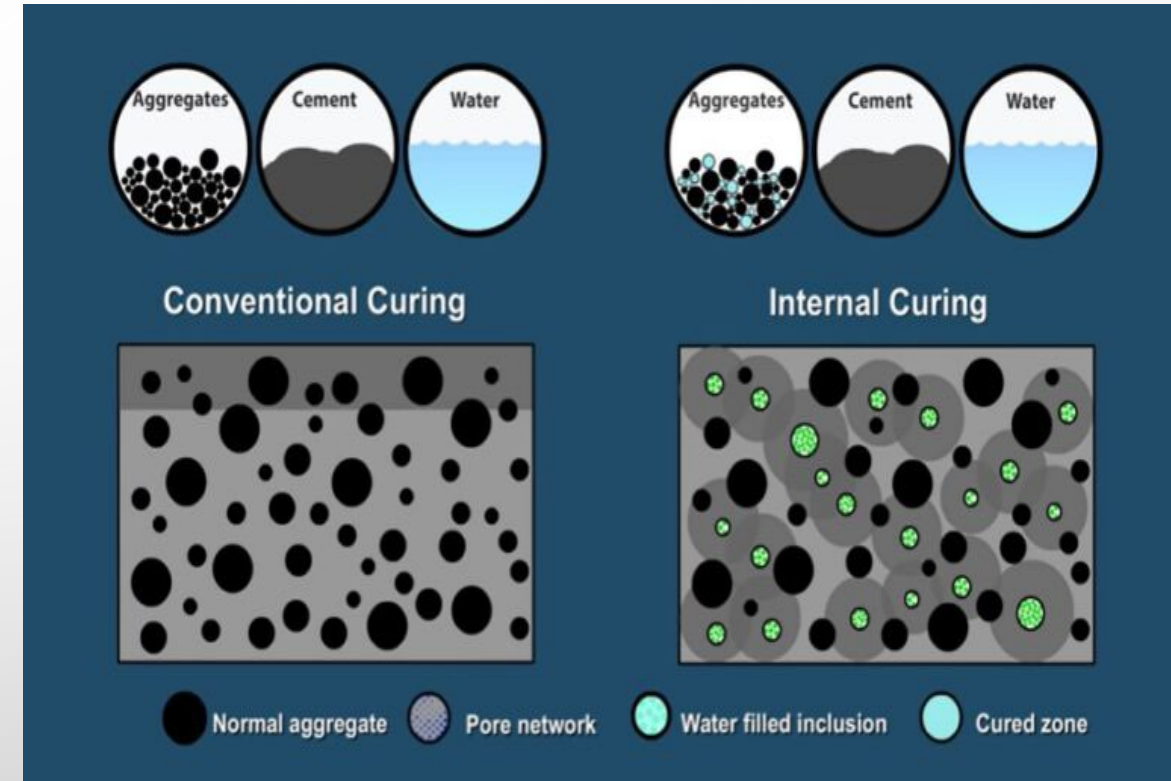
More than 30 years of extensive studies have shown that internal curing addresses:

- **Durability:** targets and mitigates the source of shrinkage cracking particularly in lower w/cm concretes, resulting in lower permeability concretes to improve durability
- **Versatility:** can be used anywhere traditional concrete is used.
- **Cost Savings:** higher-durability and reducing the need to rehabilitate or replace critical elements such as bridge decks during the design life of the bridge, resulting in life-cycle cost savings.
- **Embodied Carbon Reduction:** opportunities for increased utilization of natural, waste, or recycled SCMs without reduced performance or increased risk of cracking.



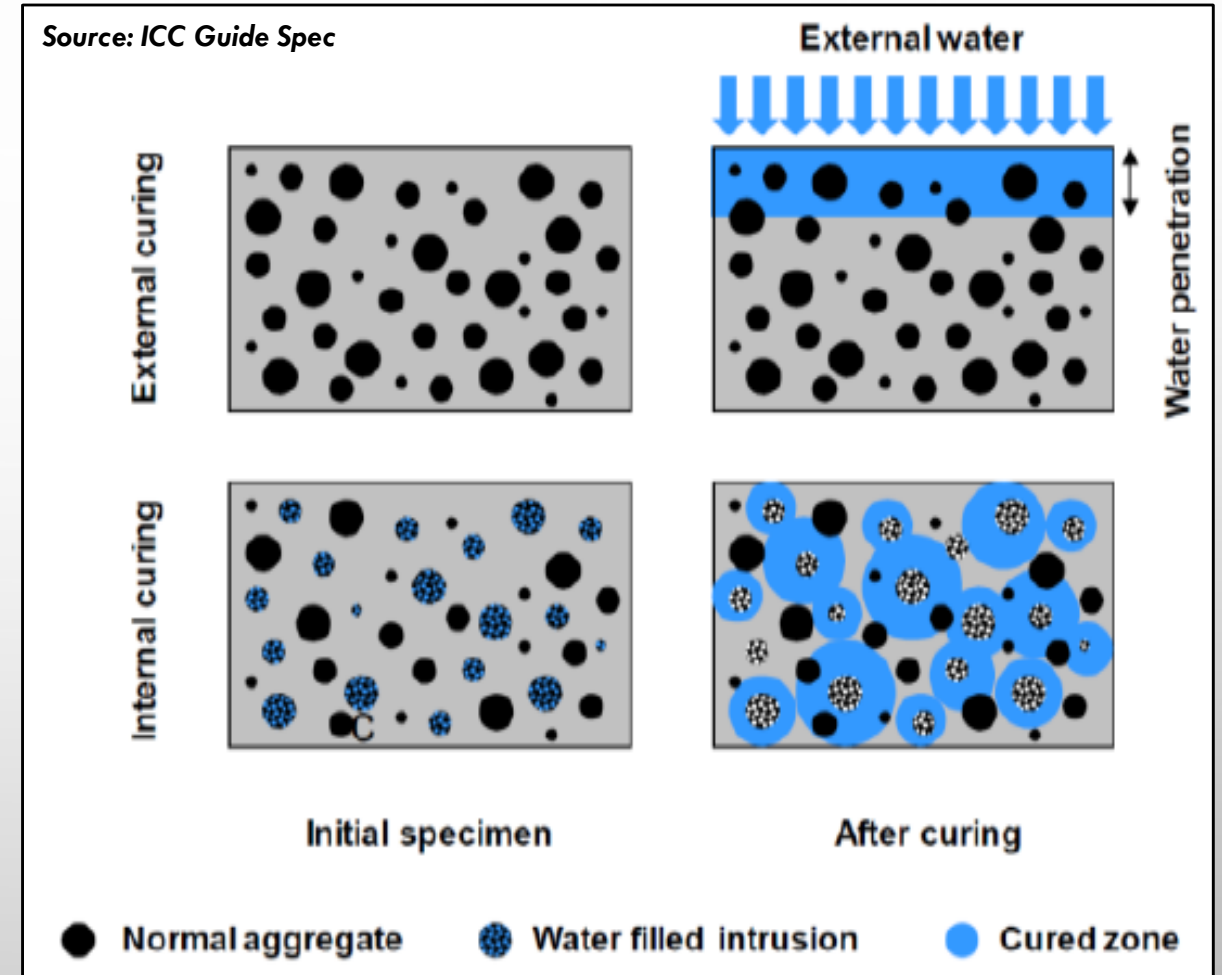
INTERNAL CURING

- Internal curing is simply curing where the water is provided from inside.
- The main difference between HPC and HPIC is Replacing some conventional fine aggregate (sand) with pre-wetted lightweight fine aggregate (LVFA).
- Water absorbed :
 - Does not contribute to the w/cm ratio
 - Remains in the LVFA during mixing and until set.
- Results in better dispersion of curing water throughout the concrete depth.



INTERNAL CURING

- At the time of set, capillary stresses draw the water out of the LFWA and cures the concrete section.
- Supplemental to, not replacement for, external curing.
- Increases the hydration of the cement and reaction of SCMs.
- Utilizing the cementitious materials more efficiently and reduces the porosity.



It is not lightweight concrete!

INTERNAL CURING DEPLOYMENT



Deployed and institutionalized by more than 15 States or transportation agencies :

- **Bridge Decks:** ~150 bridge decks
New York State, Indiana, Louisiana, North Carolina, Ohio, and Utah DOTs; Western Federal Lands Highway Division; and the Illinois State Toll Highway Authority.
- **Pavements:** Kansas and Texas DOTs and the North Texas Tollway Authority.
- **Pavement Patches:** City of West Lafayette, Indiana; Texas DOT; and Michigan municipalities.



INTERNAL CURING DEPLOYMENT

NYSDOT Experience :

- Experimental Features plan for FHWA:
12 projects, up to 20 decks using IC
- Increased durability and higher cracking resistance.
(70% reduction in early age cracking).
- No increase in cost.
- Curing period reduced from 14 to 7 days for HPIC.
- Mandatory for continuous bridges & all NYC bridges.



New York State Peer Exchange - FHWA EDC-7 EPIC2
Albany, NY | May 29-30, 2024

NJDOT IMPLEMENTATION PLAN



- Finalized HPIC guide specifications
NJDOT/ FHWA/ UTCA-NJ/ LWFA suppliers/
Concrete Plants/ Designers
- NJDOT-Approved mix plants list within the
theoretical 90-minute commute of the project
- Awarded STIC Incentive Grant - \$125,000
- NJDOT Bureau of Research awarded Rutgers
RIME Internal Curing Project

NJDOT IMPLEMENTATION PLAN

Comparative Studies : Twin Bridges HPC vs HPIC



CANDIDATE BRIDGES/PROJECTS



Project Name	Scope	County	Region	Current Phase	FDS	Award
North Munn Ave, Bridge over Rt 280	LS Super. Repl.	Essex	North	Con.	2024	2024
CR 507 (Maple Ave), Bridge over Rt 208	LS Super. Repl.	Bergen	North	FD	2025	2025
Hanover Ave (CR650), Bridge over I-287	LS Deck Repl.	Morris	North	FD	2025	2026
Prince Rodgers Ave, Bridges over Rt 287	LS Deck Repl.	Somerset	Central	FD	2025	2026
Rt 202 Bridge over North Branch of Raritan River	FS Bridge Repl.	Somerset	Central	FD	2026	2026
Rt 45 Bridge over Woodbury Creek	FS Bridge Repl.	Gloucester	South	FD	2026	2026
Rt 23 SB Bridge over NYS&W Railroad	LS Deck Repl.	Passaic	North	FD	2026	2027
Rt 124 Bridges over Passaic River	LS Deck Repl.	Morris	North	FD	2026	2028
Change Bridge Rd (CR 621), Bridge over Rt 80	LS Deck Repl.	Morris	North	CD	2027	2029
Blue Heron Rd Bridge over Rt 15	LS Deck Repl.	Sussex	North	CD	2028	2029

PROJECT IMPLEMENTATION

- Specifications (Special Provisions)
- Contract Pay Items
- Local concrete supplier coordination
- Design
- Contract Plans



SPECIAL PROVISIONS

- Section 500 (Construction)
- Section 900 (Materials)
- Performance Specification
 - Contractor Mix Design
 - Verification – Materials Testing
- Light-weight aggregate
- Test slab and trial batching
- Guide Specification developed

New Jersey
Department of Transportation

Table 903.05.04-2 Acceptance Requirements for HPIC

Test Method	Test Method	Test Method
Percent Air Entrainment ^a	AASHTO T 152	7.0 ± 1.5 (No. 8 Aggregate)
Slump (inches) ^{1,2}	AASHTO T 119	3±1
Surface Resistivity @ 56 days ^{3,4,5} (kΩ-cm, minimum)	AASHTO T 358	19
Unit Weight ⁶ (pounds per cubic foot, minimum)	AASHTO T 121	135
Compressive Strength ⁷ @ 3, 7, 28, 56 days (pounds per square inch, minimum)	ASTM C 39	4,400

1. If using a Type F or G admixture, changes to Slump and Air Content are for the HPC as follows:
 1.1 Slump: 6 ± 2 inches.
 1.2 Air Content: increase both the test value and tolerance percentages by 1.0%.

2. For slip-formed parapet, design and construct a curb with a slump of 1 ± 1/2 inch.

3. The ME will not test for the surface resistivity requirements for HPC used for other than bridge decks.

4. For surface resistivity, the ME shall mold 4 additional cylinders, taking 2 cylinders each from 2 randomly selected delivery trucks for testing at 56 days.

5. If the surface resistivity requirement has been achieved in 28 days, consider the surface resistivity acceptable. If the required surface resistivity is not achieved in 28 days, test the HPIC sample at 56 days.

6. For compressive strength testing, the initial rate for the HPIC is 6 per lot. The rate limit is 4,400 pounds per square inch.

7. If the compressive strength requirement has been achieved in 28 days, consider the strength acceptable. If the required compressive strength is not achieved in 28 days, test the HPIC sample at 56 days.

THE FOLLOWING SUBSECTION IS DELETED:

903.11 HIGH PERFORMANCE INTERNAL CURING READY-TO-PLACE CONCRETE

903.11.01 Composition

Produce HPIC as specified in 903.05.01, except for the following:

The HPIC mix design shall have total normal-weight fine aggregate volume substituted with pre-wetted lightweight fine aggregate (LWFA) that conforms to the requirements as specified in 901.06.04. The quantity of LWFA substitution shall be calculated to provide a sufficient volume of internal curing water in accordance with ASTM C1761, Appendix X1. Aggregate design absorption shall be used in the substitution calculations instead of 72-hr soaking periods. Aggregate desorption need not be considered in the substitution calculations. The maximum quantity of LWFA substitution in the mix design shall not exceed the calculated quantity required to provide internal curing water in accordance with ASTM C1761.

HPIC mixes utilizing light-weight coarse aggregate, superabsorbent polymer, or colloidal silica will not be accepted by the Department.

**Standard Specifications
for Road and Bridge Construction**

2019

MATERIALS – LIGHTWEIGHT AGGREGATE

- Expanded shales, clays, slates, slags
- High absorption capacity %
- Fine LWA (LWFA) preferred over Coarse LWA
 - Concrete sands
- ASTM C1761



Source: Hydrocure ©



MATERIALS - NJDOT QPL

Producer	Address	Quarry/Pit Location	Size	%Abs	Bulk Sp. Gr.	SSD Sp. Gr.	App. Sp. Gr.	Unit Wt.	DRW
Carolina Stalite Co.	217 Klumac Rd. P.O. Box 1037 Salisbury, NC 28144	Gold Hill,NC	It. wt. agg. for internal curing HPC	9.8	1.745	1.912	2.095	60	65
Norlite Corp.	628 South Saratoga Street Cohoes,NY 12047	Cohoes,NY	It. wt. agg. for internal curing HPC	23.2	1.587	1.831	2.100	53	59
Northeast Solite	PO Box 437 Mount Marion,NY 12456	Saugerties,NY	It. wt. agg. for internal curing HPC	12.0	2.172	1.936	1.734	62	68

LW AGGREGATE STOCKPILE MANAGEMENT

- Pre-soak with sprinkler or soaker (24-48 hrs)
 - Aggregate absorbs water
- Drain-down period (12-16 hrs):
 - Reduce surface moisture.
 - Mix & increase uniformity.
- At concrete production facility



Source: FHWA

LW AGGREGATE - MOISTURE TESTING

- Paper-towel method (ASTM C1761 / NYSDOT)
- Centrifuge Method (INDOT/Purdue Univ.)
- Performed by regional Materials personnel.
- Confirm minimum absorption % met.
- Determine excess free-moisture:
 - Batch adjustments



Source: FHWA

HPIC MIX DESIGN

- Mix conversion
- Convert existing HPC mix to internally cured
- Substitute ~30-50% of total fine aggregate (volume) with LWFA.
- LWFA substitution volume dependent on aggregate absorption/desorption properties.

MIX PROPORTIONS:		CONVERTED INTERNALLY CURED								
COMPONENT	TYPE	NAME	PRODUCER / SUPPLIER	AGGREGATE SIZE / GRADATION	RELATIVE DENSITY	ABSORPTION	WEIGHT (lb)	VOLUME (ft ³)	% MIX WEIGHT	% MIX VOLUME
Cement										
PCC1	Type III	-	Riverside Cement	-	3.150	-	570	2.900	14.3%	10.7%
PCC2	None	-	-	-	0.000	-	0	0.000	0.0%	0.0%
							Σ 570	2.90	14.3%	10.7%
Supp. Cementitious Mat.										
SCM 1	Silica Fume	-	-	-	2.200	-	25	0.182	0.6%	0.7%
SCM 2	Slag	45 Slag (E1)	EarthCEM	-	2.870	-	130	0.726	3.3%	2.7%
SCM 3	None	-	-	-	0.000	-	0	0.000	0.0%	0.0%
							Σ 155	0.91	3.9%	3.4%
Coarse Aggregate										
CA 1	Stone	-	Gibraltar Rock	#57	SSD	SSD	1320	7.568	33.1%	27.9%
CA 2	Stone	-	Gibraltar Rock	#8	2.795	0.5%	650	3.761	16.3%	13.9%
					2.770	0.5%	Σ 1970	11.33	49.5%	41.8%
Fine Aggregate										
FA 1 (NW)	Concrete Sand	-	Sahara Sand	-	SSD/WSD	SSD/WSD	501	3.047	12.6%	11.2%
FA 2 (LW)	Solite	-	-	-	2.637	0.4%	363	3.334	9.1%	12.3%
					1.745	14.4%	Σ 864	6.38	21.7%	23.6%
Water										
Mix Water	-	-	-	-	1.000	-	235	3.766	5.9%	13.9%
							Σ 235	3.77	5.9%	13.9%
Air Content										
Air Entrainment	6.5%	-	-	-	-	-	0	1.755	0.0%	6.5%
							Σ 0	1.76	0.0%	6.5%
Admixtures										
ADM. 1	Air Entraining	-	Sika Corp.	-	1.000	-	0.169	0.003	0.00%	0.01%
ADM. 2	Retarder	Plastiment	Sika Corp.	-	1.000	-	0.945	0.015	0.02%	0.06%
ADM. 3	HRWR	Viscocrete 2100	Sika Corp.	-	1.000	-	1.890	0.030	0.05%	0.11%
ADM. 4	None	-	-	-	-	-	0.000	0.000	0.00%	0.00%
							Σ 3.004	0.05	0.1%	0.2%
BATCH TOTAL							Σ 3797	27.09	1.00	
							lb	ft ³	yd ³	

MIX DESIGN PROPERTIES:		CONVERTED INTERNALLY CURED	
Plastic Unit Weight =	140	lb/ft ³	[Theoretical unit weight of plastic concrete mixture]
Total Weight Cementitious =	725	lb	[Weight of cement + SCMs]
Sack Content (Cement) =	6	sack	[# of 94 lb Cement sacks per CY]
Sack Content (Cement + SCM) =	8	sack	[# of 94 lb Cement sacks per CY, including SCMs]
Gallons Water =	28.2	gal	[Gallons mix water required]
W/C =	0.41	-	[Water-Cement ratio, by weight]
W/CM =	0.32	-	[Water-Cementitious Materials ratios, by weight]
Paste Volume Fraction =	0.28	-	[Fraction of volume of paste (cement, SCMs, and water) to total batch volume]
Mortar Volume Fraction =	0.58	-	[Fraction of volume of mortar (cement, SCMs, fine aggregate, water, and entrained air) to total batch volume]
SCM/CEMENT =	0.27	-	[Ratio of SCMs to Cement, by weight]
Sand / Total Aggregate =	0.36	-	[Ratio of sand to total aggregate, by volume]

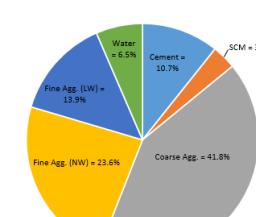
Volumetric Distribution:

- Cement = 10.7%
- SCM = 3.4%
- Coarse Agg. = 41.8%
- Fine Agg. (NW) = 11.2%
- Fine Agg. (LW) = 12.3%
- Water = 13.9%
- Air = 6.5%

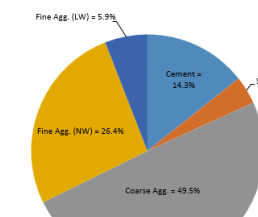
Weight Distribution:

- Cement = 14.3%
- SCM = 3.9%
- Coarse Agg. = 49.5%
- Fine Agg. (NW) = 12.6%
- Fine Agg. (LW) = 9.1%
- Water = 5.9%

Mix Proportions - Volumetric Distribution
Conventional HPC

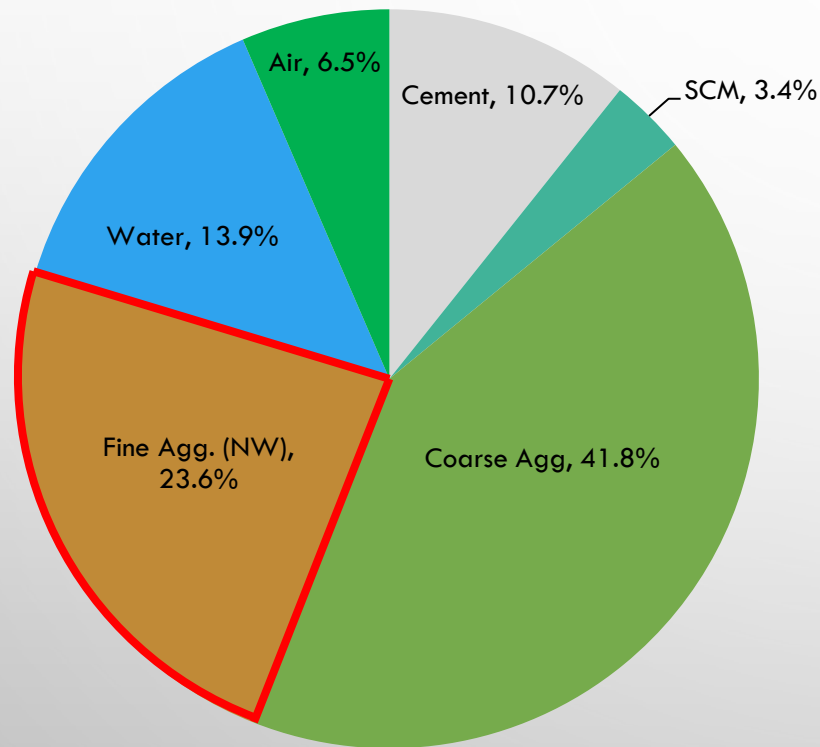


Mix Proportions - Weight Distribution
Conventional HPC

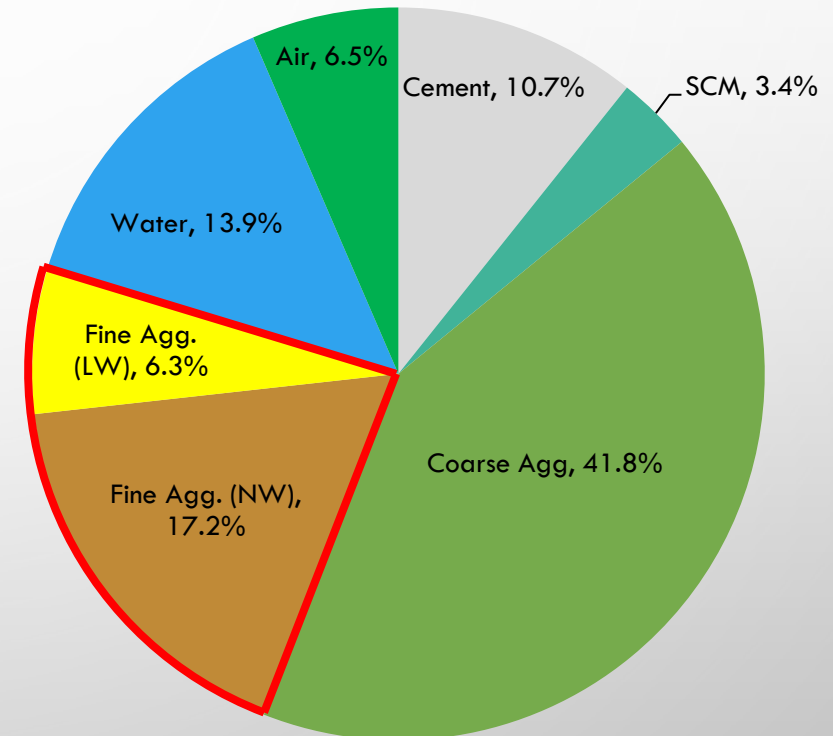


HPIC MIX CONVERSION

(VOLUMETRIC DISTRIBUTION)



HPC



HPIC
(27% LWFA) –Norlite (23% Abs.)

TRIAL BATCH & TEST SLAB

- Demonstrate contractor capabilities and provide experience to contractor & Department.
- Same stockpile management, batching, delivery and placement, and acceptance testing methods on test slab as production deck.
- Separate contract pay item.



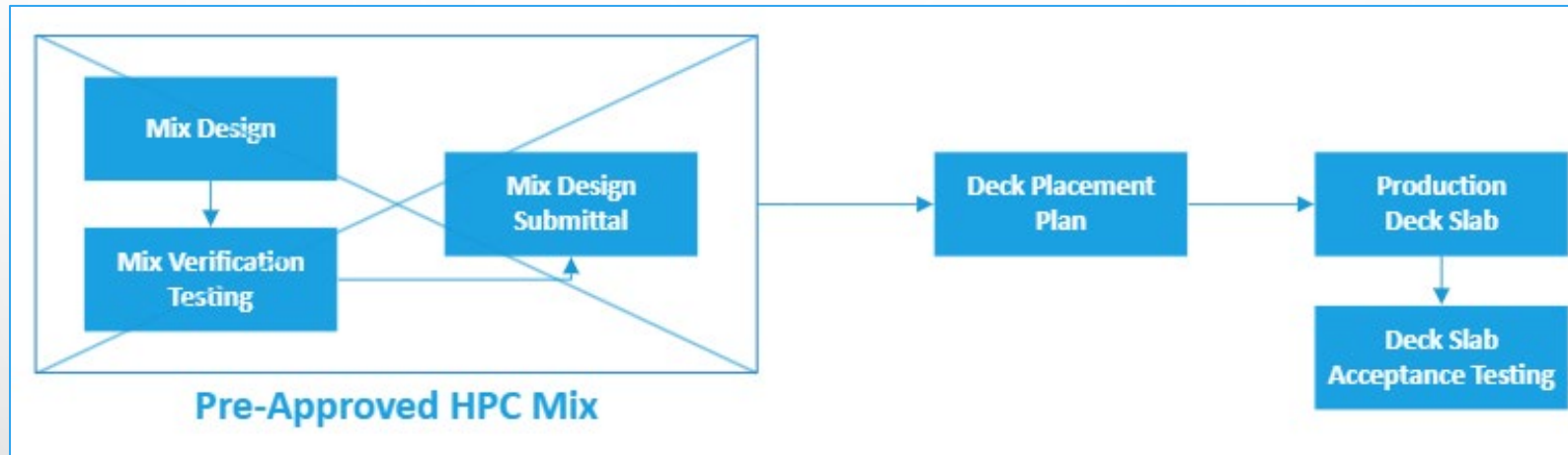
PRODUCTION & CONSTRUCTABILITY

- Batching:
 - LWFA stockpile management (soaking & moisture adjustments)
 - additional material needs to be batched
- Same placement/finishing methods as HPC:
 - Comparable workability and placement
 - Pumpable
 - Slump
- External Curing:
 - Reduced duration from 14 to 7 days possible

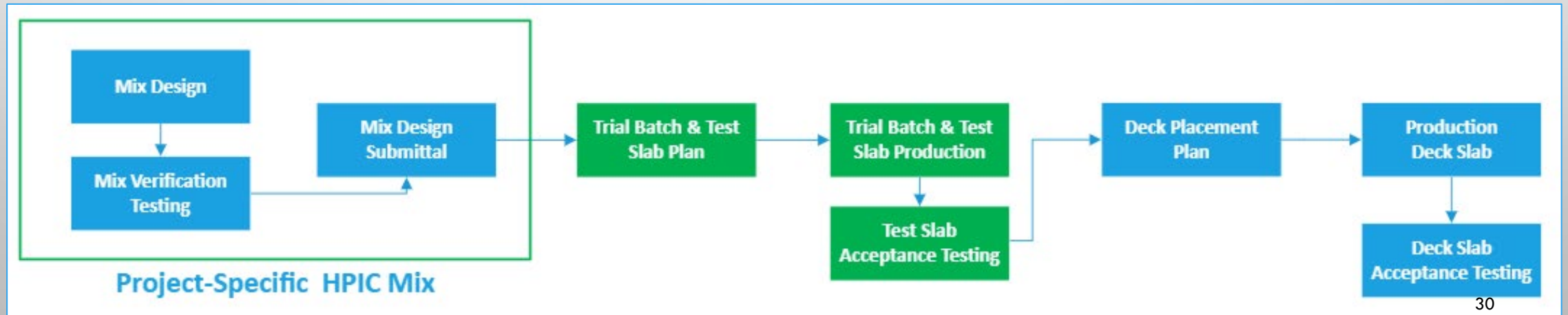


CONSTRUCTION PROCESS FLOW-CHART

HPC

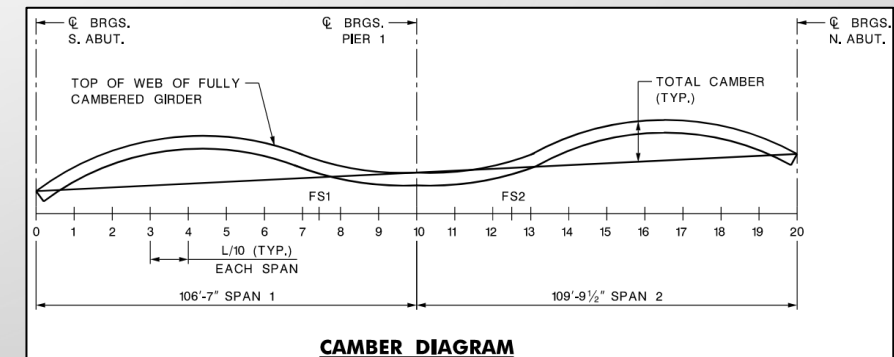
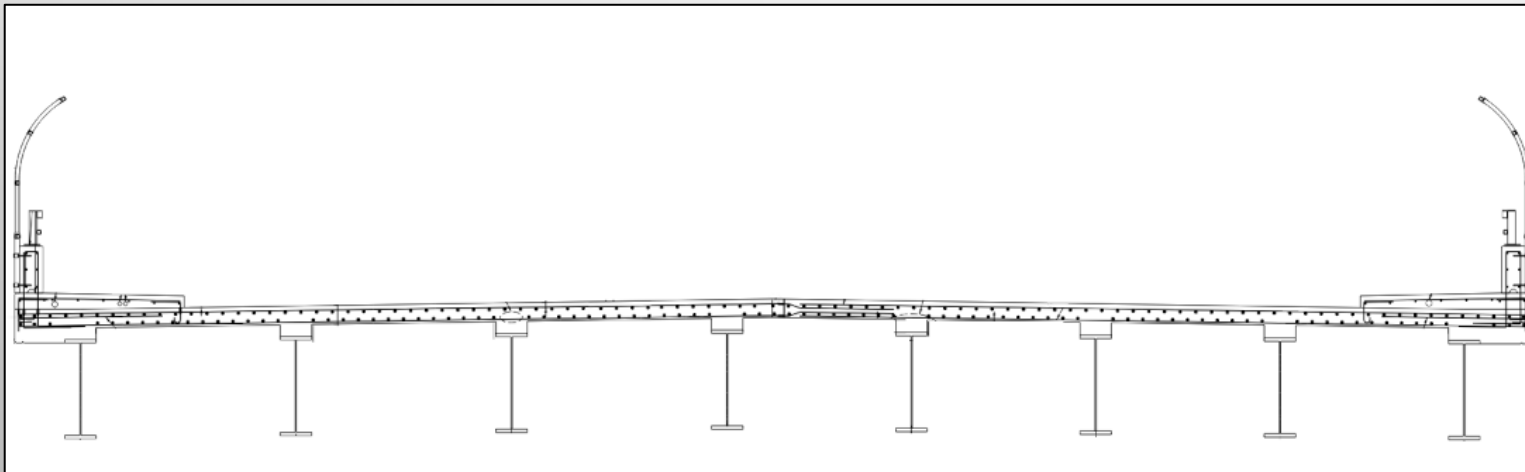


HPIC



DESIGN

- Minimal effect on concrete design properties
- Minor improvement in compressive strength
- Minor reduction in unit weight & modulus
 - Consider deck weight, girder camber, stiffness effects on design



$$E_c = 120,000 K_1 w_c^{2.0} f_c'^{0.33}$$

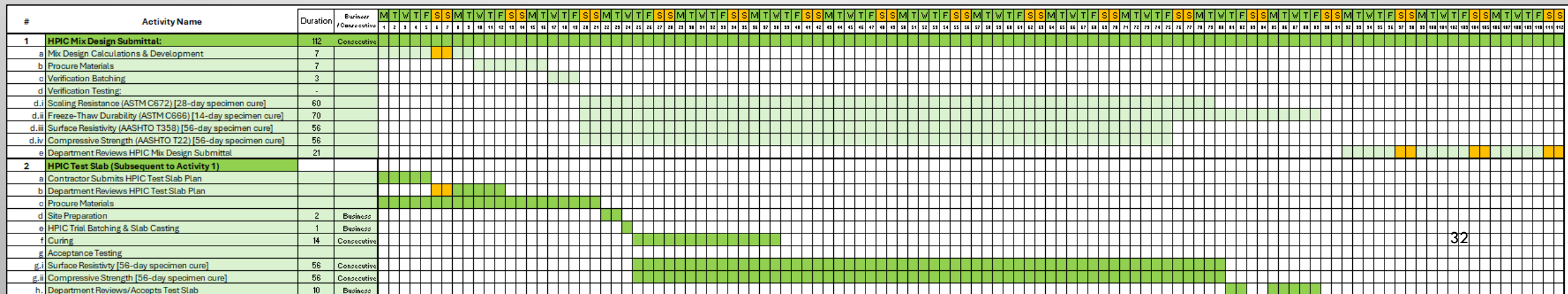
EFFECTS ON PROJECT

Design Phase:

- Designer additional effort:
 - Confirm design properties
 - Incorporate guide specifications
 - Coordinate with local concrete plants

Construction Phase:

- Additional HPIC construction costs:
 - new mix design and verification testing (25k)
 - trial batch & test slab (25k)
 - unit cost for production (per CY) (+20-40% unit cost increase)
- Schedule – additional ‘up-front’ construction duration: mix verification testing, trial batch, test slab (~6 months)



HPIC Submittal, Mix Design, Verification Testing, & Test Slab Schedule

NORTH MUNN AVE. OVER I-280

- FIRST PILOT PROJECT
- STATUS:
 - MIX DESIGN - COMPLETE ✓
 - VERIFICATION BATCHING - COMPLETE ✓
 - VERIFICATION TESTING (SURFACE RESISTIVITY, COMPRESSIVE STRENGTH, FLEXURAL STRENGTH, SHRINKAGE) - COMPLETE ✓
 - VERIFICATION TESTING (SCALING, FREEZE-THAW) – PENDING...
 - TRIAL BATCH & TEST SLAB – PENDING...
 - PRODUCTION – PENDING...



CHALLENGES

- Costs – Initial vs Life Cycle
- Specifications – Restrictions
- Concrete Plants:
 - Experience and willingness to produce IC mixes
 - Concerns with restrictive HPC acceptance testing
- Materials Availability / Supply Chain:
 - Ample availability, but coordination required
- Economy of Scale & Standardization
- Awareness/Education



NEXT STEPS..

- CONCRETE PLANT OUTREACH PROGRAM - SUMMER 2025
- HPIC WORKSHOPS - SUMMER 2025
- CENTRIFUGE TRAINING PROGRAM - FALL 2025
- UPDATE SPECIFICATIONS
- ASSESS PILOT PROJECTS
- MONITOR PERFORMANCE



TEAM EFFORT

- FHWA
- NJDOT:
 - CONSTRUCTION & MATERIALS
 - BRIDGE ENGINEERING
 - PROJECT MANAGEMENT
 - CAPITAL PROGRAM SUPPORT
- RUTGERS
- HNTB
- INITIAL INVESTIGATIONS TO CONSTRUCTION WITHIN 2 YEARS





QUESTIONS?